

Shared Anchoring of Marine Renewable Energy Devices Utilizing Monopiles

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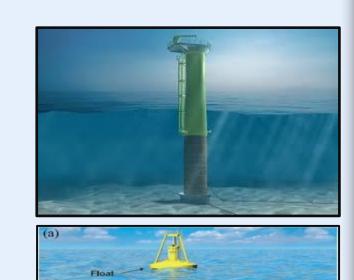
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Introduction

Kitty Hawk and Wilmington **offshore** wind lease areas have a combined potential power generating capacity of nearly 4 GW. There is an opportunity to deploy Marine Hydrokinetic (MHK) devices integrated with Offshore Wind Turbines (OWT).

- Optimizing the cost of installation and operation.
- > Increasing the energy yield.
- contributing to optimal use of natural resources.

Monopile foundation is the most likely to be used in the relatively shallow waters of North Carolina offshore wind lease area. Common standards for analyzing monopiles are for long and slender elements employed in oil and gas industry.



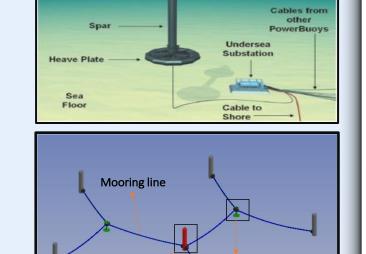


Fig. 1 OWT monopile (Saipem SA), floating point absorber (Xie et al., 2013), and a schematic co-located system.



Fig. 2. Monopiles for Taiwan's Formosa 1 offshore wind farmPhoto.

Large monopile wall thickness is required for drivability, hence under design loads excess capacity exists. Therefore, shared anchoring is possible. However:

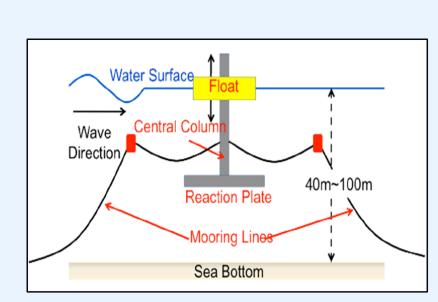
- > No information in Standards are related to **Shared Anchoring.**
- > Lack of guidance to address the impact of pipe size and lower flexibility monopiles on the foundation system response under loading.

Objective

- > A Systematic assessment of excess capacity of OWT monopiles as a function of the embedded length/diameter, while meeting Ultimate Limit State (ULS), and Serviceability Limit State (SLS).
- > Investigating the effect of monopile wall thickness/diameter on the natural frequencies and damping of the system with and without **shared anchoring** approach.

Modeling and Methods

The reference model RM3 device, which is a twobody floating point absorber (FPA) is considered as the MHK device. To keep the floating device in position, the point absorber is connected to a mooring system, per Fig. 3. The RM3 device is designed for water depths of 40-100 m. For the case of North Carolina offshore wind, the water depth is less than 40m, the device was scaled at 1/3 scale.



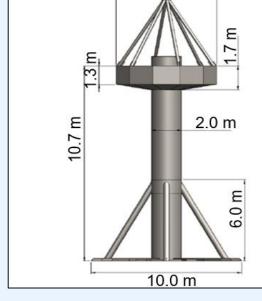


Fig 3. The schematic side view of RM3 (Neary et al., 2014).

Fig.4. RM3 scaled down device.

Table 1. Model and full-scale parameters.

Parameter	unit	Scaling Coeff	Model scale	Full scale
Mass	ton	α^3	25	680
Rated power	kW	$lpha^{3.5}$	6	286

ANSYS-AQWA software is used to simulate the effect of waves and wind on the tension in the mooring system under different dynamic loading scenarios per Table 2.

Table 2. Summary of wind and wave characteristics for loading scenarios considered in the project.

	Symbol	Unit	Normal and	Extreme wind
			operational condition	load scenario
Wind speed	U	m/s	12.94	20.1
Wave height	Н	m	5.30	10.0
Wave period	Т	S	8.10	11.2

In Fig. 5, a 600 seconds simulation using a time step of 0.01 seconds under the action of extreme load scenario is presented. The maximum force in cable one occurs at t = 149 sec and is equal to 958 kN. The corresponding forces in cables 2 and 3 are 494 kN. These forces are applied on the monopile as a support for the **co-located** system.

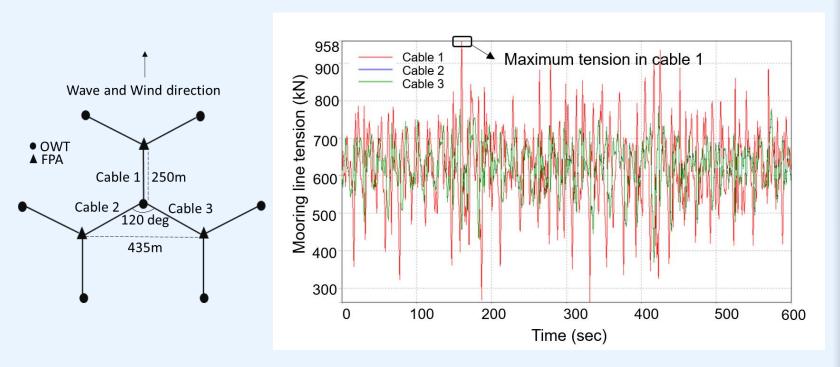


Fig.5. Schematic plan view of co-located system and mooring lines tension.

Modeling and Methods (contd.)

PLAXIS 3D is employed to perform numerical static and dynamic simulation of OWT monopile. A parametric study is performed for the **Gunfleet Sands OWT** (Table 3) With the Siemens SWT-3.6-107 3.6 MW as the wind turbine.

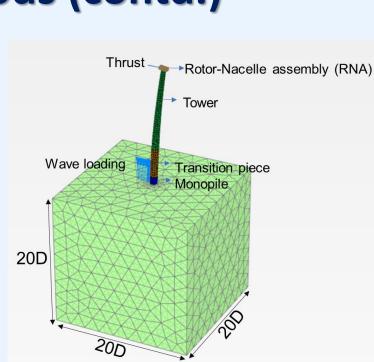


Fig. 6. Numerical 3D model in PLAXIS.

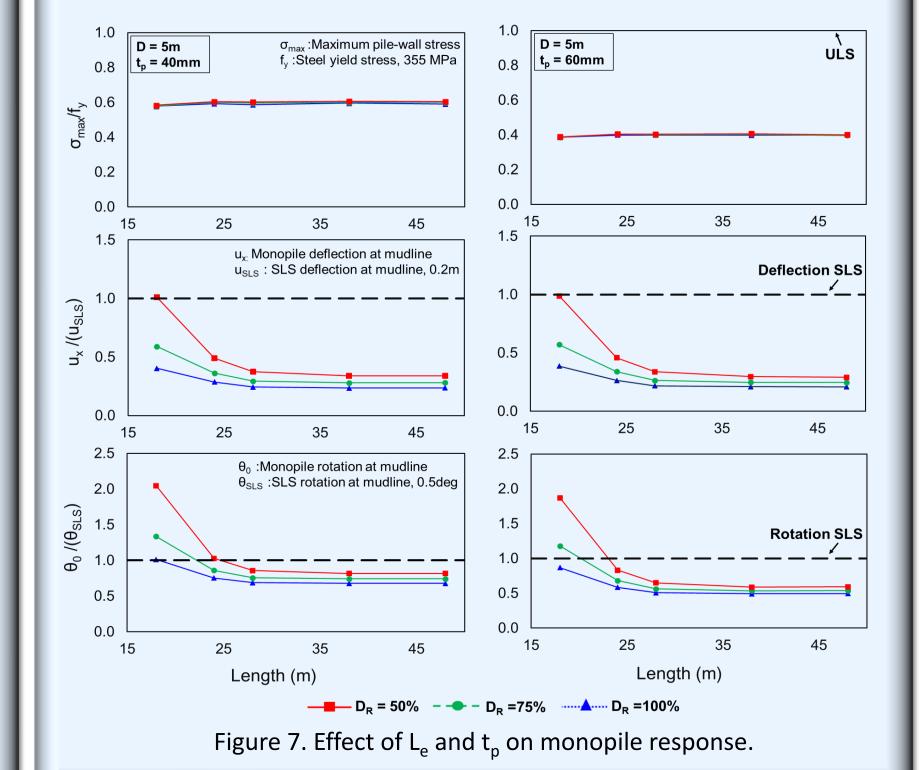
Table 3. Wind turbine data (after Arany et al., 2016)

Parameter	Symbol	Value	Unit
Hub height from MSL	Z_hub	73	m
Depth of water	d_{w}	18	m
Mass of the rotor	m_{RT}	243	Tons
Pile diameter	D	5	m
Pile wall thickness	tp	50	mm
Embedded length	Le	38	m
Pile Young's modulus	E_p	200	GPa
Pile material yield stress	f_{yp}^{\cdot}	355	MPa
Rotor operational frequency	f_{1p}	0.077 to 0.2	Hz
Blades turning frequency	f_{3p}	0.231 to 0.6	Hz
Soil relative density	D_R^{r}	75	%

Results

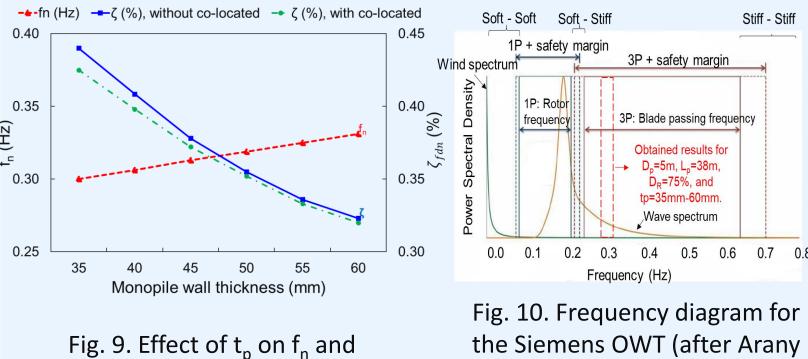
Monopiles with L_e of 28-48m and t_p > 40mm meet **ULS** and **SLS** design criteria and have extra capacity to carry additional loads from WEC.

Increasing t_p from 35 to 55mm increases f_n from 0.3Hz to 0.325Hz. Wind turbine operates within blade turning frequency (f3P) outside the soft-stiff zone.



Results (contd.) σ_{max}:Maximum pile-wall stress _γ:Steel yield stress, 355 MPa D = 5m $D_{R} = 50\%$ $D_{R} = 75\%$ x. Monopile deflection at mudline **Deflection SLS** . :Monopile rotation at mudline 9_{SIS} :SLS rotation at mudline, 0.5deg Rotation SLS Monopile wall thickness (mm)

Fig. 8. Effect of t_n on the response of the co-located system.



et al., 2015b).

Conclusion

damping

- \triangleright OWT monopiles with $L_e = 28-48m$ and $t_p > 40mm$ meet **ULS** and **SLS** specifications with and without shared anchoring approach.
- > The co-located configuration system does not significantly change the $\mathbf{f_n}$ and foundation damping.
- > The Gunfleet Sands turbine operates within blade turning frequency. These analyses suggest changes in tower dimensions would be required to bring the f_n into the soft superstructure-stiff substructure region.

Acknowledgment

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